

# Rainfall-runoff Modelling in a Semi-urbanized Catchment using Self-adaptive Fuzzy Inference Network

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**Keywords:** Rainfall-runoff Modelling, Neuro-fuzzy Systems, SaFIN, ANFIS, SWMM, ARX.

**Abstract:** Conventional neuro-fuzzy systems used for rainfall-runoff (R-R) modelling generally employ offline learning in which the number of rules and rule parameters need to be set by the user in calibration stage. This makes the rule-base fixed and incapable of being adaptive if some rules become inconsistent over time. In this study, the Self-adaptive Fuzzy Inference Network (SaFIN) is used for R-R application. SaFIN benefits from an adaptive learning mechanism which allows it to remove inconsistent and obsolete rules over time. SaFIN models are developed to capture the R-R process in two catchments including Dandenong located in Victoria, Australia, and Sungai Kayu Ara catchment in Selangor, Malaysia. Models' performance are then compared with the ANFIS, ARX, and physical models. Results show that SaFIN outperforms ANFIS, ARX, and physical models in simulating runoff for both low and peak flows. This study shows the good potential of using SaFIN in R-R modelling application.

## 1 INTRODUCTION

Rainfall-runoff (R-R) modelling as one of the important topics in hydrology is focused on better understanding of the rainfall-runoff process which is necessary to address some of hydrological problems such as urban water management and flood forecasting. In addition to physical and conceptual models, there is a third group of R-R models known as system theoretic models which involves a direct mapping (linear/non-linear) between the inputs and output data (Minns and Hall, 1996). System theoretic models do not use the knowledge of the system's parameters directly but instead formulate its own set of parameters based purely on the dataset. Examples of such models are regression-based models, Artificial Neural Networks (ANN), and Neuro-Fuzzy Systems (NFS) (Xiong et al., 2001, Rajurkar et al., 2002, Sajikumar and Thandaveswara, 1999). NFS are hybridizations of fuzzy set theory and neural networks which provide the mapping of input-output data with varying degrees of non-linearity. NFS learning can generally be classified as either offline learning or online learning systems. Offline or batch learning formulates model parameters based on a

static dataset, whereas online learning enables models to sequentially update its parameters during each timestep of the training data. The benefit of online learning models is that it allows a model to inherit a dynamic training approach where the model parameters evolves sequentially as new data becomes available, enabling the model to capture time varying properties within the system; whereas offline learning models requires a retraining process of the entire dataset merged with new data to achieve similar results, resulting in greater computational time and complexity.

NFS models with offline learning such as Adaptive Network-based Fuzzy Inference System (ANFIS) are extensively used in R-R modelling (Nayak et al., 2004, Nayak et al., 2005, Remesan et al., 2009, Mukerji et al., 2009, Talei et al., 2010b, Talei et al., 2010a, Bartoletti et al., 2017, Zakhrouf et al., 2015). The major drawback of a model such as ANFIS is its offline learning algorithm where the number of rules is pre-set by the user and remains fixed. In real-world applications, a reliable R-R model should be able to dynamically capture time-varying properties within a system through a continuous process of updating and reiterating its

model parameters. To date, not many studies have been made on addressing adaptability through online learning adaptation in R-R modelling. In recent literature, several authors have attempted incorporating online learning into various R-R modelling applications and has generally shown improvement in modelling performance (Hong, 2012, Luna et al., 2007, Talei et al., 2013, Ashrafi et al., 2017, Chang et al., 2016). In this study, Self-adaptive Fuzzy Inference Network (SaFIN) (Tung et al., 2011) is adopted and applied in developing a R-R model. SaFIN is known for its capability of being self-adaptive which enable the learning mechanism to add and remove rules automatically. This study aims to investigate the capabilities of using SaFIN as a R-R model while comparing its performance with ANFIS and a physical benchmark model known as Storm Water Management Model (SWMM).

## 2 SELF-ADAPTIVE FUZZY INFERENCE NETWORK (SaFIN)

SaFIN is a self-organizing neural fuzzy system with incremental online learning capabilities developed by Tung et al (2011). SaFIN is a fully data-driven model capable of formulating and maintaining a consistent rule-base automatically. SaFIN was developed to address several issues faced in previously existing models such as inconsistencies within the rulebase, the need for prior knowledge, and addressing the stability-plasticity tradeoff. SaFIN consists of a five-layer multilayer perceptron (MLP) network which employs the neural network-based gradient descent approach to fine-tune the parameters of its membership functions. In conventional neuro-fuzzy systems, the fuzzy clusters and fuzzy rule base require

initialization through the knowledge of human experts. To address this issue, SaFIN employs two learning mechanisms: (1) self-organizing clustering, and (2) self-automated rule generation (Tung et al, 2011). Through the self-organizing clustering technique, the numbers, positions, and spreads of fuzzy labels are self-determined from the training dataset. This clustering technique of SaFIN is known as Categorical-Learning Induced Partitioning (CLIP). The main motivation for using CLIP is the fact that it is a tailored approach for addressing the stability-plasticity dilemma of NFS models. CLIP draws inspiration from the behavioural category learning process exhibited by humans whereby categorical learning builds up from a basic high-contrasting level of distinction to a low-contrasting categorical distinction. CLIP represents these categorical distinctions as Gaussian membership functions where the parameters  $\alpha$  and  $\beta$  allows direct control of these fuzzy labels. Membership functions transfer the crisp values of input space to fuzzy values. Although there are several mathematical functions that can be used for this purpose, Tung et al (2011) suggested using Gaussian membership function in CLIP.

Figure 1 shows the fuzzy partitioning process of CLIP during (A) initialization and (B) the addition process of second cluster. During initialization, the first membership function is centered over the input value while covering over the entire domain in each input-output dimension as shown in (A.a – A.b). At this stage, parameter  $\alpha$  determines the minimum threshold of the membership function, where the membership value at any point within the domain is at least  $\alpha$ . This implies that a high  $\alpha$  value describes a wider spread and a greater global significance of the fuzzy label. CLIP then progresses to regulate the newly made fuzzy label to maintain semantic prevalence as shown in A.c. In B.a, when a new data point is present in SaFIN, a similarity measure is

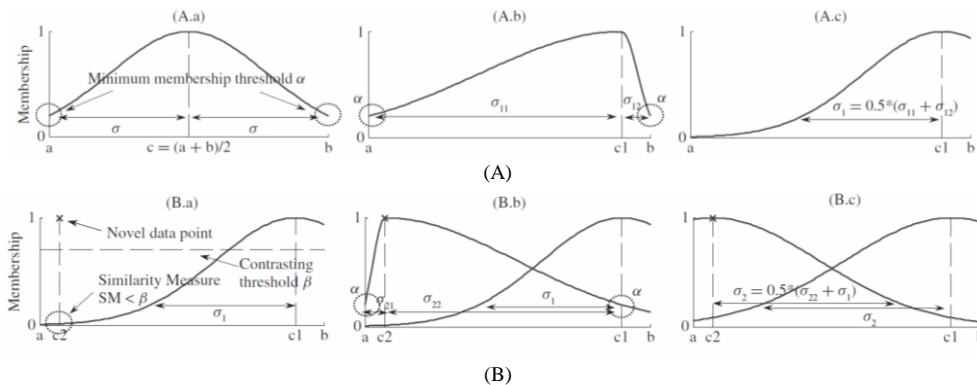


Figure 1: CLIP Clustering Technique (Tung et al., 2011) (A) Initialization process; (B) Additional process of second cluster.

calculated for each existing cluster to determine the fuzzy cluster that best relates to the new input. Parameter  $\beta$  is defined as the contrasting threshold between the new data point and the best-matched fuzzy label to determine the novelty of the new data when compared against existing fuzzy labels. If the similarity measure metric is greater than  $\beta$ , no new labels will be added into the system since a similar label already exists within the system. Conversely, if the similarity measure metric is less than  $\beta$ , the new data point is deemed to be novel and CLIP proceeds to the addition of a new cluster as shown in B.b.

then added into the rule-base if determined to be novel. Weights are also assigned to each rule as the allocated weight is important in depicting each rules significance while allowing the system to remove any low impact or conflicting rules. Consistency checks are performed upon rule-base formation for inconsistent rule-base, which can be rules with similar precedent conditions but with varying outcomes. When inconsistency is found, the rule with the lower weightage will be removed. This method provides the rule pruning capability in SaFIN where inconsistent and obsolete rules is removed over time.

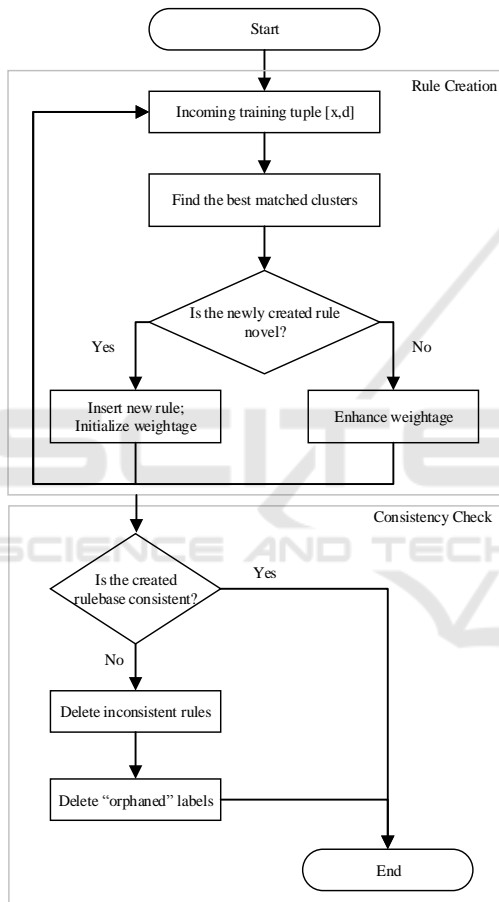


Figure 2: Flowchart of self-automated rule generation mechanism implemented in SaFIN.

SaFIN also employs a self-automated rule generation mechanism which formulates and updates the rule base accordingly over time as depicted in Figure 2. Upon achieving fuzzy partitioning of data with CLIP, the rule base is ready to be formulated. Rule generation runs in two stages, rule creation and rule consistency check. For each incoming training tuple, a novelty check is conducted between the new data and its best matched fuzzy cluster, a new rule is

### 3 METHODOLOGY

#### 3.1 Study Site and Data Used

Dandenong catchment (Catchment 1) with an area of about 272 km<sup>2</sup> is chosen as the study site which is located in South East of Melbourne, Australia (See Figure 3). The primary creek in this catchment is the Dandenong creek which originates from the Dandenong Ranges National Park and discharges into Port Phillip Bay via both Mordialloc Creek and Patterson River. Although farmlands as well as some forest pockets remain in the catchment, approximately 45% of the land has been overcome by urbanization. Also, industrial activities are carried extensively in large areas of the catchment. Eleven years of daily rainfall and river discharge readings from January 2005 to December 2015 from stations Dandenong, Rowville, and Heathmont are used in this study where Rowville and Heathmont are the two upstream stations with Heathmont having the highest elevation.

Sungai Kayu Ara river basin (Catchment 2) is situated in a largely flattened urban landscape in Selangor, Malaysia, and covers an area of 23.22 km<sup>2</sup> (See Figure 4). The river basin is located within the equatorial zone which is subjected to northeast and southwest monsoon seasons. Annual mean rainfall within the region is more than 2000mm while average daily temperatures ranges from 25°C to 33°C. The annual average evaporation rate for the basin is estimated at 4 to 5mm per day, while mean monthly relative humidity falls within 70% to 90%. The basin consists 10 rainfall station and 1 river discharge station. 40 rainfall-runoff events with 10-minutes time series were extracted from the rainfall stations spanning between March 1996 and July 2004.



Figure 3: Schematic layout of Dandenong catchment.

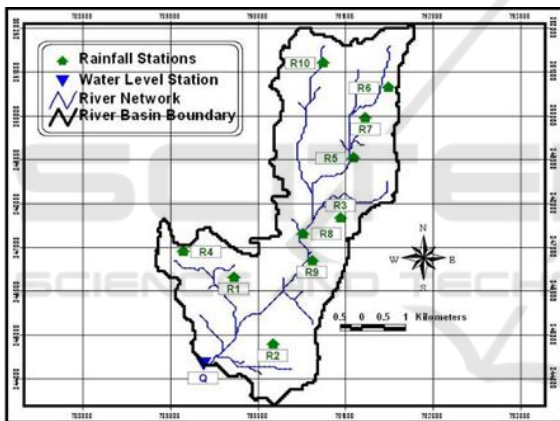


Figure 4: Schematic layout of Sungai Kayu Ara river basin.

### 3.2 Physically-based Model Used

#### 3.2.1 Storm Water Management System (SWMM)

Storm Water Management System (SWMM) is a dynamic rainfall-runoff simulation model developed by the United States Environmental Protection Agency (US EPA) used in conducting runoff quantity and quality simulations. SWMM conceptualizes physical elements of a watershed system into a standard set of modelling objects where rain gauges and sub-catchments are the principal objects used to model the rainfall-runoff process. Each sub-catchment is further subdivided into impervious and pervious regions for simulating precipitation, evaporation and infiltration losses. Using kinematic

wave equation, SWMM simulates the runoff based on the physical routing of runoff through a system of pipes and channels through a collective sub-catchment area resulted by precipitation. The kinematic equation is typically used in rainfall-runoff modelling in which the model solves the continuity equation along with a simplified form of the momentum equation, allowing variations in spatial and temporal flows within a conduit.

SWMM is one of the most widely used model in a variety of hydrologic applications which includes urban sewer planning, rainfall-runoff modelling, and stormwater quality modelling. The model allows flexibility of adjusting over 150 different constants and coefficients which are physical dimensions, impervious observations, soil properties and pipe characteristics.

#### 3.2.2 Hydrologic Engineering Center - Hydrologic Modelling System (HEC-HMS)

HEC-HMS is a lumped conceptual model in hydrological applications. It attempts to simulate the physical processes within the rainfall-runoff response of a river basin system to a precipitation input through conceptualizing the entire river basin as a system that is interconnected by hydrologic and hydraulic components like river basins, streams and reservoirs. HEC-HMS is designed to be light in computational complexity but flexible for a wide range of geographic areas with different environment and climates. The model includes many of the processes involved in water circulation in the basin, such as, precipitation, evaporation or infiltration. As such, the model is widely used in many studies involving water resources.

HEC-HMS requires pre-processing through HEC-GeoHMS (Geospatial Hydrologic Modelling). HEC-GeoHMS is an extension of ArcGIS which is specifically designed for surface delineation and producing the required geospatial data for HEC-HMS hydrologic modelling. A surface Digital Elevation Model (DEM) was used to extract drainage paths and watershed boundaries to represent the hydrologic structure used for simulating the watershed response to precipitation. Results produced by HEC-GeoHMS is then extracted and exported into HEC-HMS for watershed hydrologic modelling.

### 3.3 Adaptive Network-based Fuzzy Inference System (ANFIS)

ANFIS combines the reasoning capabilities of fuzzy



systems with the learning mechanism of neural networks. ANFIS was first developed by Jang (1993) who implemented the Takagi-Sugeno fuzzy rules in a five-layer neural network. Figure 5 shows the typical structure of an ANFIS model for the case of 2 inputs.

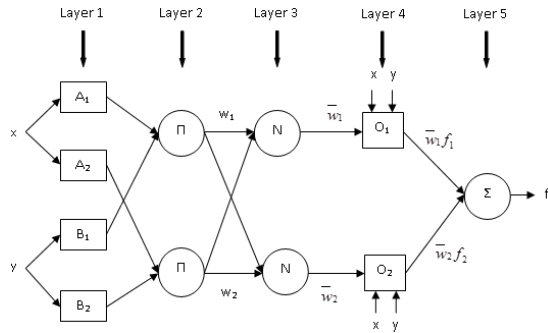


Figure 5: Typical ANFIS structure for 2 inputs.

Further details about each layer and the corresponding variables can be found in Talei et al. (2010b). ANFIS has been successfully used in several engineering applications including rainfall-runoff modelling; therefore, it has been chosen as a benchmark model in this study for comparison purposes.

### 3.4 Input Data Selection and Model Development

In Catchment 1, 11 years rainfall-runoff time series were split into 2 datasets. The first 8 years was used as training (calibration) dataset while the remaining 3 years of the data was used as validation dataset. The input selection was conducted on training data set where totally 6 rainfall antecedents of  $R_D(t)$ ,  $R_D(t-1)$ ,  $R_R(t)$ ,  $R_R(t-1)$ ,  $R_H(t)$ , and  $R_H(t-1)$  and 4 discharge antecedents of  $Q_R(t)$ ,  $Q_R(t-1)$ ,  $Q_H(t)$ ,  $Q_H(t-1)$  were considered as candidate inputs. It is worth mentioning that  $R_D$ ,  $R_R$ ,  $R_H$  are rainfall at Dandenong, Rowville, and Heathmont stations, respectively while  $Q_R$ ,  $Q_H$  are upstream discharge at Rowville and Heathmont stations, respectively;  $t$  is the present time and  $t-1$  is considered as a one-day lag. For Catchment 2, 40 event-based data were split into 12 training events and 28 testing events. The rainfall-runoff dataset consists of a total of 10 rainfall antecedents and 1 river discharge output,  $Q(t)$ . The 10 rainfall antecedents ranges from  $R1$  to  $R10$ , where the position of each respective rainfall station is shown in Figure 4.

An input selection analysis was applied on both catchments rainfall and discharge antecedents in order to determine the choice of inputs for modelling.

As with most data driven models, the selection of inputs is necessary to ascertain inputs that are better associated with the discharge consequent to attain greater efficacy during modelling. A hybridization of both correlation analysis and mutual information analysis proposed by Talei and Chua (2012) is adopted in this study to select the inputs. This approach prioritizes the inputs that have high correlation with the output while possessing low mutual information with other inputs. The Pearson correlation coefficient is obtained by:

$$CC(x, y) = \frac{\sigma_{xy}}{\sigma_x \sigma_y} = \frac{\sum_{i=1}^n (x_i - \bar{x})(y_i - \bar{y})}{\sqrt{\sum_{i=1}^n (x_i - \bar{x})^2} \times \sqrt{\sum_{i=1}^n (y_i - \bar{y})^2}} \quad (1)$$

in which  $\sigma_{xy}$  is the covariance between variables  $x$  and  $y$ ;  $\sigma_x$  and  $\sigma_y$  are the standard deviations of  $x$  and  $y$ , respectively;  $\bar{x}$  and  $\bar{y}$  are the average values of  $x$  and  $y$ , respectively, and  $n$  is the number of data points. On the other hand, mutual information of two variables  $x$  and  $y$ ,  $MI(x, y)$  is calculated by:

$$MI(x, y) = \frac{1}{2} \log \left( \frac{\sigma_x^2 \sigma_y^2}{|\sigma_{xy}|} \right) \quad (2)$$

where  $\sigma_x^2$  and  $\sigma_y^2$  are the variance of the two variables  $x$  and  $y$ , respectively and  $\sigma_{xy}$  is the covariance between variables  $x$  and  $y$ .

### 3.5 Performance Criteria

In order to evaluate the models' performance, four different statistical measures are considered in this study.

#### 3.5.1 Nash-Sutcliffe Coefficient of Efficiency (CE)

Coefficient of efficiency can be obtained by:

$$CE = 1 - \frac{\sum_{i=1}^n (Q_{Obs,i} - Q_{Sim,i})^2}{\sum_{i=1}^n (Q_{Obs,i} - \bar{Q}_{Obs})^2} \quad (3)$$

where  $Q_{Obs,i}$  and  $Q_{Sim,i}$  are the observed and simulated discharge values (in  $m^3/s$ ) for the  $i$ th data point, respectively;  $\bar{Q}_{Obs}$  is the average value of the observed discharge while  $n$  is the total number of data points. It is worth mentioning that CE varies in the

domain of  $(-\infty, 1]$  and is used to assess the goodness-of-fitness between observed and simulated discharge values of this study.

### 3.5.2 Coefficient of Determination ( $R^2$ )

Coefficient of determination which measures the degree of co-linearity between observed and simulated values, varies in the range of  $[0, 1]$ . Value of 1 indicates the perfect positive association while the value of zero indicates no association. This measure can be calculated by:

$$r^2 = \frac{\left[ \sum_{i=1}^n (Q_{Obs,i} - \bar{Q}_{Obs})(Q_{Sim,i} - \bar{Q}_{Sim}) \right]^2}{\left[ \sum_{i=1}^n (Q_{Obs,i} - \bar{Q}_{Obs})^2 \times \sum_{i=1}^n (Q_{Sim,i} - \bar{Q}_{Sim})^2 \right]} \quad (4)$$

where  $Q_{Obs,i}$  and  $Q_{Sim,i}$  are the observed and simulated discharge values (in  $m^3/s$ ) for the  $i$ th data point, respectively;  $\bar{Q}_{Obs}$  and  $\bar{Q}_{Sim}$  are the average value of the observed and simulated discharge, respectively, while  $n$  is the total number of data points.

### 3.5.3 Root Mean Squared Error (RMSE)

RMSE accords extra importance on the outliers in the data set and is therefore biased towards errors in the simulation of high flow rates.

$$RMSE = \sqrt{\frac{\sum_{i=1}^n (Q_{Sim,i} - Q_{Obs,i})^2}{n}} \quad (5)$$

where  $Q_{Obs,i}$  and  $Q_{Sim,i}$  are the observed and simulated discharge values (in  $m^3/s$ ) for the  $i$ th data point, respectively;  $n$  is the total number of data points.

### 3.5.4 Mean Absolute Error (MAE)

MAE is the average of all deviations from the original data regardless of their sign. This parameter does not allocate any weight to errors in extreme values. MAE can be calculated by:

$$MAE = \frac{\sum_{i=1}^n |Q_{Sim,i} - Q_{Obs,i}|}{n} \quad (6)$$

where  $Q_{Obs,i}$  and  $Q_{Sim,i}$  are the observed and simulated discharge values (in  $m^3/s$ ) for the  $i$ th data

point, respectively;  $n$  is the total number of data points.

### 3.5.5 Relative Peak Error (RPE)

Peak estimation in rainfall-runoff modelling is a very sensitive tasks since this measure is dealing with extreme events. In this study, RPE is adopted to evaluate the models' capability in predicting peak values. RPE is defined as:

$$RPE = \frac{|(Q_{p,Obs}) - (Q_{p,Sim})|}{Q_{p,Obs}} \quad (7)$$

where  $Q_{p,Obs}$  and  $Q_{p,Sim}$  are the observed and simulated peak discharge. Values closer to zero indicate better estimation of peak flows.

## 4 RESULTS AND DISCUSSION

### 4.1 Dandenong Catchment (Catchment 1)

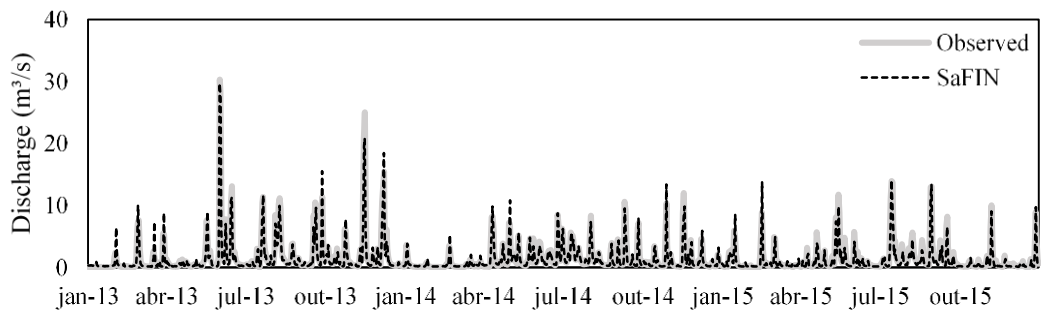
Based on input selection analysis the best combination of inputs was found to be of  $R_D(t-1)$ ,  $Q_R(t)$ ,  $Q_H(t)$ . Both SaFIN and ANFIS model was calibrated using the same training data and input combination. In addition, SWMM was also calibrated using 1 arc-second resolution DEM data as well as rainfall data from 9 different rainfall gauges. Further comparisons were made through benchmarking against results obtained from the autoregressive model with exogenous inputs (ARX) model. ARX is a linear regression model for input-output mapping.

In R-R modelling, ARX model output,  $Q(t)$  is assumed to be related to rainfall antecedents,  $R(t-i)$  and past outputs  $Q(t-i)$  by the following formula:

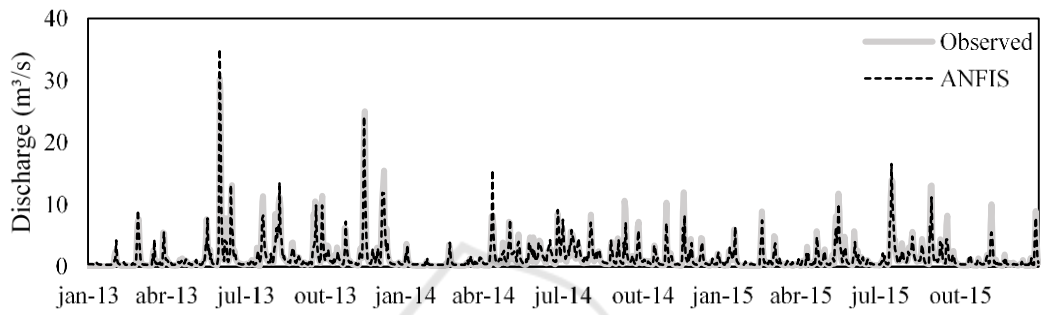
$$Q(t) = -\sum_{i=1}^{n_a} a_i Q(t-i) + \sum_{j=1}^{n_b} b_j R(t-n_k-j+1) + e(t) \quad (8)$$

where  $n_a$  and  $n_b$  are the number of past outputs and inputs respectively,  $n_k$  is the delay associated with each input,  $e(t)$  is the true error term; and  $a_i$  and  $b_j$  are model parameters to be optimized. To determine the optimal model parameters, model fit is evaluated using three residual statistics which are RMSE, Akaike Information Criterion (AIC) (Akaike, 1974) and Bayesian Information Criterion (BIC) (Rissanen, 1978). AIC and BIC are denoted by:

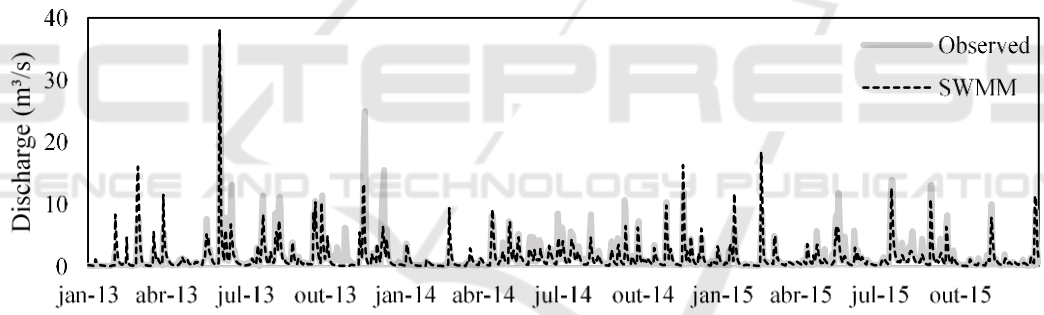
$$AIC = n_{i=0} \ln(RMSE) = 2n_p \quad (9)$$



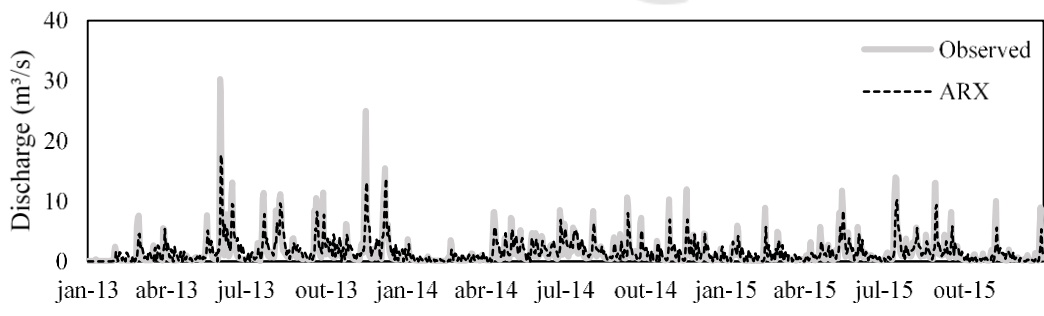
(a)



(b)



(c)



(d)

Figure 6: Observed versus simulated hydrograph in Catchment 1 by (a) SaFIN, (b) ANFIS, (c) SWMM and (d) ARX.

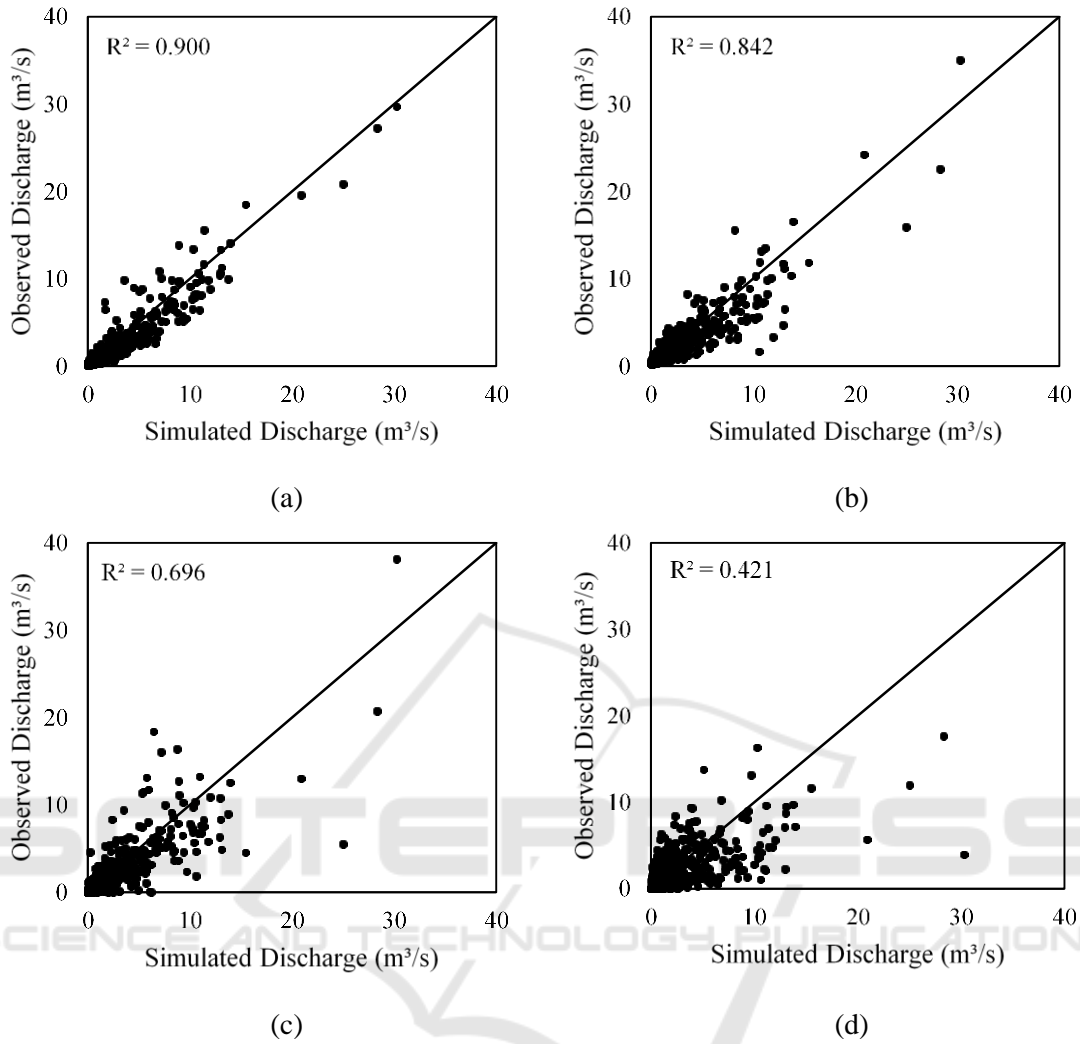


Figure 7: Scatterplots of observed versus simulated discharge in Catchment 1 by (a) SaFIN, (b) ANFIS, (c) SWMM and (d) ARX.

$$\text{BIC} = n_{i=0} \ln(\text{RMSE}) = n_p \ln(n_{i=0}) \quad (10)$$

where  $n_{i=0}$  is the number of input-output patterns and  $n_p$  is the number of model parameters. ARX was employed with through varying range of values for parameters  $n_a, n_b, n_k$ .

The model performance of all 4 models were then compared using several performance metrics including coefficient of efficiency (CE),  $R^2$ , RMSE, and MAE as provided in Table 1.

Table 1: Performance of different models in Catchment 1.

Model	CE	$R^2$	RMSE	MAE
SaFIN	0.893	0.900	0.893	0.468
ANFIS	0.841	0.842	1.087	0.527
SWMM	0.686	0.696	1.532	0.671
ARX	0.417	0.421	1.174	0.550

As it can be seen, SaFIN was able to outperform ANFIS, SWMM and ARX models for all performance indices. Although SaFIN and ANFIS models used data from 3 rainfall stations compared to the 9 that was used to develop SWMM, both models were able to outperform SWMM. However, it should be noted that both SaFIN and ANFIS had the advantage of having upstream discharge data as inputs which contributes to performance improvement. For further comparison, the observed hydrograph is compared with the simulated ones by SaFIN, ANFIS, SWMM and ARX as shown in Figure 6. As can be seen, all models were able to simulate various ranges of flow in the testing dataset. To evaluate the performance of the models in peak estimation, the RPE metric was calculated for peak discharge values greater than  $10 \text{ m}^3/\text{s}$  (total 27 peaks).



Figure 7 shows the scatterplots for each of the 4 model simulations. The scatterplots produced by SaFIN and ANFIS appear to have an almost similar spread in simulating low flows while ANFIS shows more underestimations and overestimations for higher flows values. Whereas the SWMM scatterplot shows a wider spread when compared to SaFIN and ANFIS.

Figure 8 shows the boxplots of the RPE values obtained from SaFIN, ANFIS, SWMM and ARX. As can be seen, SaFIN has the lowest median value and the least range of errors when compared to the other models followed by ANFIS and SWMM model. ARX was the worst among these four models in the peak estimation.

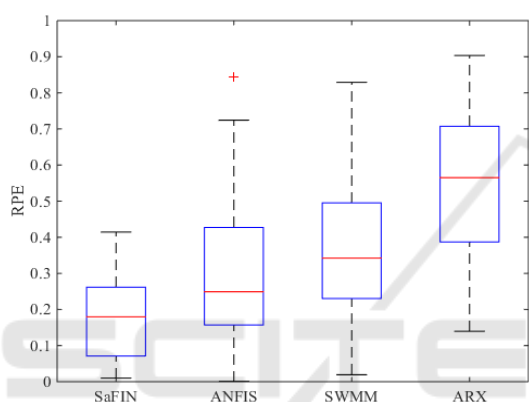


Figure 8: RPE boxplots for SaFIN, ANFIS, SWMM and ARX in Catchment.

#### 4.2 Sungai Kayu Ara River Basin (Catchment 2)

SaFIN and ANFIS were both trained and tested using inputs  $R_1(t-7)$ ,  $R_3(t-8)$ ,  $R_5(t-7)$  and  $Q(t-1)$  that were obtained from input selection analysis. It is worth mentioning that  $R_i$  refers to the  $i$ th rainfall station. The results were compared against the ones obtained by HEC-HMS from a study conducted by Alaghamand et al. (2010). Additionally, ARX was used as an additional benchmark to represent a linear regression model. The averaged performance criteria across 28 testing datasets for all 4 models were compared and shown in Table 2.

Table 2: Performance of different models in Catchment 2.

Model	CE	R <sup>2</sup>	RMSE	MAE
SaFIN	0.851	0.868	3.201	3.021
ANFIS	0.824	0.829	3.425	3.275
HEC-HMS	0.743	0.862	3.813	3.261
ARX	0.423	0.501	8.552	8.794

From the averaged results, SaFIN outperformed ANFIS, HEC-HMS and ARX in all performance measures. ANFIS marginally underperformed as compared to SaFIN, while the linear regression model fails to model the highly non-linear nature of rainfall-runoff modelling. Although both neuro-fuzzy models were capable of performing better than the physical model and linear regression models, it is worth noting that SaFIN and ANFIS were trained and tested using discharge antecedents with a lag of one timestep. Figure 9 shows the boxplots of performance criteria across 28 testing datasets simulated in catchment 2 by the 4 models of this study. As it can be seen, SaFIN boxplots show a consistently low spread across all performance criteria. Additionally, SaFIN was able to simulate peak discharge values more accurately and consistently when compared to the other models.

Figure 10 shows the scatterplots of observed versus simulated discharge for all 4 models. SaFIN shows a relatively good performance in low and high discharge values while having a larger spread in simulating mid-peak discharges. Both ANFIS and HEC-HMS show less consistency in simulating the different categories of flow in this catchment when compared to SaFIN. The simulated discharge obtained from ARX model was consistently poor for both low and high flows.

## 5 CONCLUSIONS

SaFIN R-R model with rule-pruning mechanism was able to outperform an offline NFS model, ANFIS, ARX model, and two physical models SWMM and HEC-HMS in two different catchments in terms of several goodness-of-fit indices. Moreover, it was found that SaFIN significantly outperform ANFIS, ARX, and the two physical models in peak estimation. This study showed the great potential for using SaFIN in Rainfall-Runoff modelling application. SaFIN's ability in updating its rule-base was found as its major strength when compared to the conventional NFS models with offline learning.

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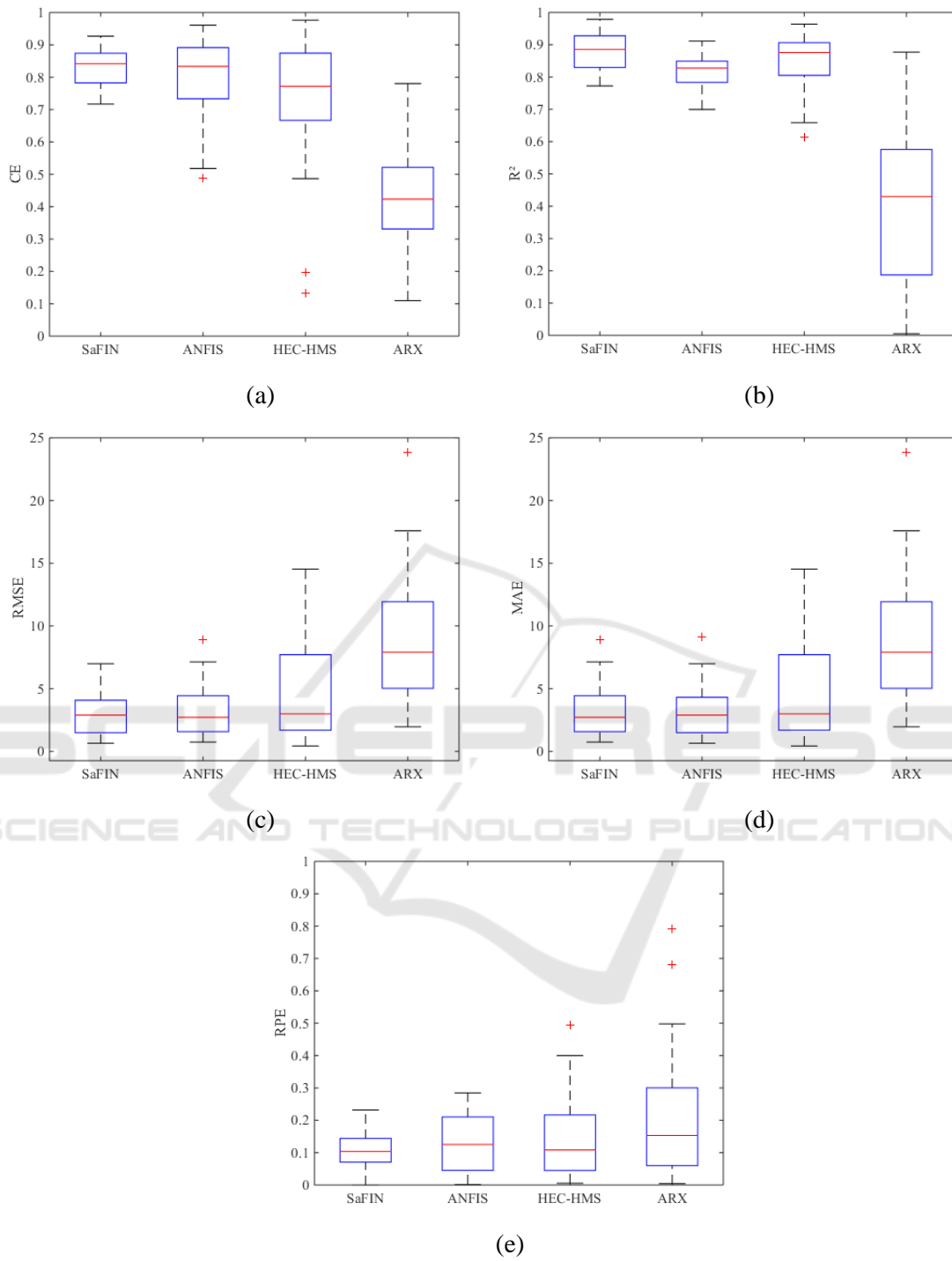


Figure 9: Boxplots of performance criteria: (a) CE, (b) R<sup>2</sup>, (c) RMSE, (d) MAE and (e) RPE for SaFIN, ANFIS, HEC-HMS and ARX models in Catchment 2.

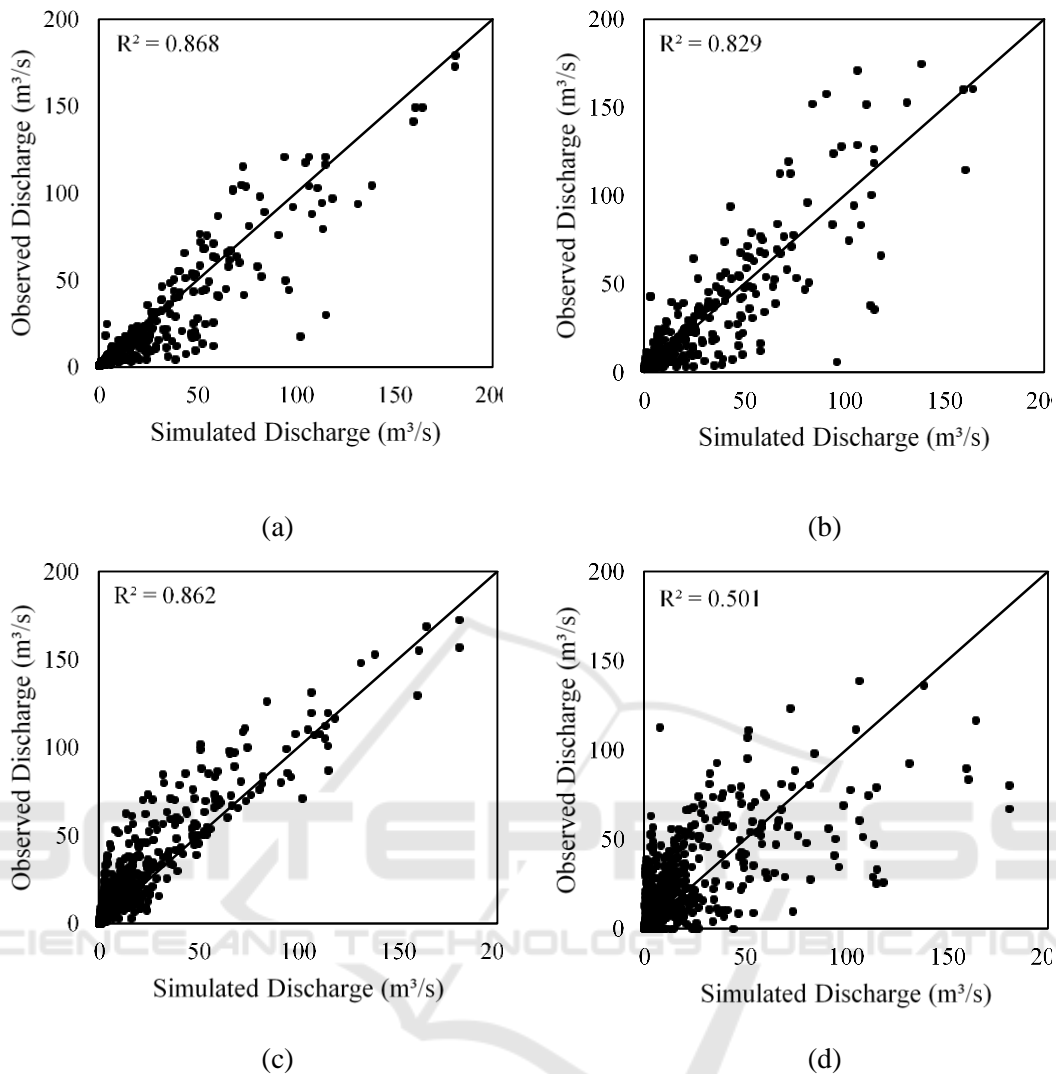


Figure 10: Scatterplots of observed versus simulated discharge in Catchment 2 by (a) SaFIN, (b) ANFIS, (c) HEC-HMS and (d) ARX.

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